



**YILDIZ TECHNICAL UNIVERSITY  
CIVIL ENGINEERING DEPARTMENT  
CONSTRUCTION MATERIAL  
DIVISIONS**

**CONSTRUCTION MATERIALS /3rd LABORATORY REPORT: CONCRETE MIX  
DESIGN & THE FRESH AND HARDENED PROPERTIES OF CONCRETE**

**Name-Surname:**

**Group:**

**Student No.:**

**Session:**

**3.1. CONCRETE MIX DESIGN**

**Table 3. 1** Physical properties of the constituents of concrete

| Materials                            | Specific gravity (kg/dm <sup>3</sup> ) | Mix ratio (%) | Water absorption (%) | Moist content (%) |
|--------------------------------------|--|---------------|----------------------|-------------------|
| Fine Aggregate 1: Natural Sand       | 2.67                                   | 20            | 1.2                  | 3.8               |
| Fine Aggregate 2: Crushed Sand       | 2.69                                   | 25            | 0,9                  | 2                 |
| Coarse Aggregate: Crushed Stone No.1 | 2.71                                   | 55            | 0.5                  | 0.2               |
| Cement: CEM I 42,5 R                 | 3.1                                    |               |                      |                   |
| Superplasticizer                     | 1.15                                   |               |                      |                   |

**Table 3. 2** Recommended limit values for concrete mix and its properties

| Exposure Class | Min Strength Class | Max w/c ratio | Min cement dosage kg/m <sup>3</sup> | Min Air content, % |
|----------------|--------------------|---------------|-------------------------------------|--------------------|
| XC3            | C30/37             | 0.55          | 280                                 | -                  |

Calculate the amounts of the components to be used in the concrete mixture according to the preliminary test results and material properties given below.

- ✓ Water/cement ratio and cement dosage are 0.42 and 355 kg/m<sup>3</sup>, respectively.
- ✓ Air content of the fresh concrete was obtained as % 1.6%.
- ✓ The solid content of the superplasticizer is 35%.
- ✓ The desired slump class was obtained by using 0.9% superplasticizer.

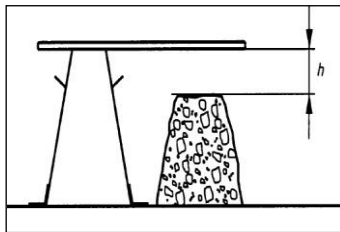
**Calculations:**

**Table 3. 3** Amount of constituents for 1m<sup>3</sup>

| Materials          | Amount of constituents for SSD condition (kg) | Amount of constituents for moist aggregate (kg) |
|--------------------|---|---|
| Cement             |   |   |
| Water              |   |   |
| Natural Sand       |   |   |
| Crushed Sand       |   |   |
| Crushed Stone No.1 |   |   |
| Superplasticizer   |   |   |

### **3.2 FRESH STATE PROPERTIES OF CONCRETE**

#### **3.2.1 Slump Test (TS EN 12350-2)**



Measured slump value (h) is .....cm.

According to this result, the slump class of fresh concrete is ..... defined in TS EN 206.

**Figure 3. 1** Slump test

#### **3.2.2 Fresh Density ( $\beta$ ) (TS EN 12350-6)**



**Figure 3. 2** Concrete mold with dimensions of 150×150×150 mm<sup>3</sup>

**Table 3. 4** The density of fresh concrete

| No.            | W(kg) | $\beta$ (kg/m <sup>3</sup> ) |
|----------------|-------|------------------------------|
| 1              |       |                              |
| 2              |       |                              |
| 3              |       |                              |
| <b>Average</b> |       |                              |



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**CONSTRUCTION MATERIALS/ 3rd LABORATORY  
REPORT: HARDENED CONCRETE TESTS**

Name-Surname:

Group:

Student No.:

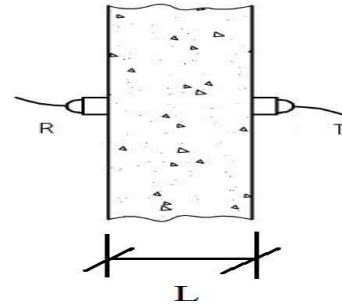
Session:

**3.3. Non-Destructive Tests**

**3.3.1. Ultrasonic Pulse Velocity (UPV) Test (TS EN 12504-4)**

**Table 3.5.** UPV test results

| No.             | Time ( $\mu$ s) | Length (mm) | UPV (mm/ $\mu$ s) |
|-----------------|-----------------|-------------|-------------------|
| 1               |                 | 150         |                   |
| 2               |                 | 150         |                   |
| 3               |                 | 150         |                   |
| <b>Average:</b> |                 |             |                   |

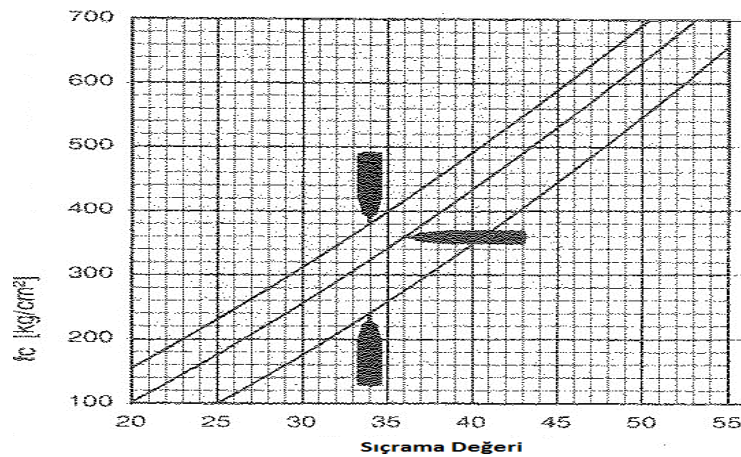


**Table 3.6.** Evaluation of concrete by determining the UPV  
(Whitehurst, E.A., "Soniscopes Tests Concrete Structures", Journal of ACI)

| UPV (mm/ $\mu$ s) | Concrete Quality | ✓ |
|-------------------|------------------|---|
| > 4,5             | Excellent        |   |
| 3,5 – 4,5         | Good             |   |
| 3,0 – 3,5         | Questionable     |   |
| 2,0 – 3,0         | Poor             |   |
| < 2,0             | Very Poor        |   |

**3.3.2. Rebound Hammer (Schmidt Hammer) Test (TS EN 12504-2)**

| Rebound Number |  |
|----------------|--|
| 1              |  |
| 2              |  |
| 3              |  |
| 4              |  |
| 5              |  |
| 6              |  |
| 7              |  |
| 8              |  |
| 9              |  |
| 10             |  |
| 11             |  |
| 12             |  |
| <b>Average</b> |  |



**Figure 3.3.** Relation between compressive strength and rebound number (1 kg/cm<sup>2</sup>=0.0981 MPa)

**Estimated Compressive Strength:  $f_{c,tah} \approx \dots\dots\dots$  MPa**

### 3.4. DESTRUCTIVE TESTS

#### 3.4.1. Compressive Strength Tests (TS EN 12390-3)

**Table 3.7.** Test results and conformity criteria for the compressive strength classes (TS-EN 206)

| No.   | $P_c$ (kN) | $f_c$ (MPa) | Evaluation  |
|---|------------|-------------|---|
| 1   |            |             | $f_{cm} \geq f_{ck} + 4,0$ (MPa)<br>$f_{c,min} \geq f_{ck} - 4,0$ (MPa) |
| 2   |            |             |   |
| 3   |            |             |   |
| <b>Average (<math>f_{c,m}</math>):</b>                    |            |             |   |
| <b>Minimum Individual value (<math>f_{c,min}</math>):</b> |            |             |   |

**Table 3.8.** Compressive strength classes for normal-weight and heavy-weight concrete (TS EN 206)

| Basınç dayanımı sınıfı | En düşük karakteristik silindirik dayanımı | En düşük karakteristik küp dayanımı |
|------------------------|--|-------------------------------------|
|                        | $f_{ck,sil}$<br>N/mm <sup>2</sup>          | $f_{ck,küp}$<br>N/mm <sup>2</sup>   |
| C 8/10                 | 8  | 10                                  |
| C 12/15                | 12   | 15                                  |
| C 16/20                | 16   | 20                                  |
| C 20/25                | 20   | 25                                  |
| C 25/30                | 25   | 30                                  |
| C 30/37                | 30   | 37                                  |
| C 35/45                | 35   | 45                                  |
| C 40/50                | 40   | 50                                  |
| C 45/55                | 45   | 55                                  |
| C 50/60                | 50   | 60                                  |
| C 55/67                | 55   | 67                                  |
| C 60/75                | 60   | 75                                  |
| C 70/85                | 70   | 85                                  |
| C 80/95                | 80   | 95                                  |
| C 90/105               | 90   | 105                                 |
| C 100/115              | 100  | 115                                 |

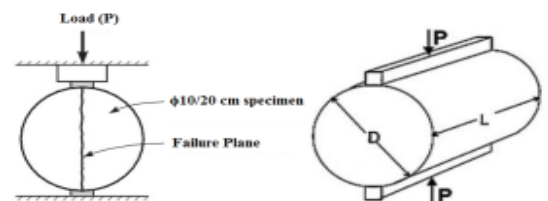
#### Calculations:

According to the test results, the compressive strength class is determined as ..... Hence, this concrete *conforms / does not conform* the minimum strength class requirement (C30/37) for exposure class XC3 that was considered during the mix design.

#### 3.4.2. Splitting-Tensile Strength Test (TS EN 12390-6)

**Failure Load:  $P_s =$  ..... kN**

**$f_t =$  ..... MPa**



**Figure 3.4** Split-tension test